CONTRACT ADDENDUM

ADDENDA NO.: 001

DATE ISSUED: MARCH 22, 2024

BID DATE: FRIDAY APRIL 5, 2024

BID TIME: 2 PM ET

BID LOCATION: City of Dalton Finance Department

CONTRACTOR ACTION:

1. Acknowledge receipt of the first addendum by writing in "Addenda No. 1" on page 0200-3 of bid submission.
2. Replace bid form (page 18 of 63) in bid package with 'Exhibit A' included within Addenda No. 001.
3. Please refer to 'Exhibit C' below for the official plan holder's list for this project.

INTERPRETATIONS:

Responses by the City of Dalton follow the questions in red font.

1. What is the asphalt removal item on the original bid form pertaining to?
    - Given that the City intends on removing existing asphalt pavement, asphalt removal line item has been removed from the bid form, and is reflected in 'Exhibit A' in Addenda No. 001.
2. Do the changes in the fence elevations need to be stepped down/up, or incorporate an angled transition?
    - The top of the decorative fence shall be stepped 90 degrees vertically where elevation changes are required along the retaining wall.
3. Is the Contractor responsible for provisions for temporary fencing to keep pedestrian traffic out?
    - Contractor shall be responsible for installing provisions to close off the construction area form pedestrians. To clarify this for bidding purposes, line item no. 30 "TEMPORARY FENCING" on revised bid form has been incorporated to provide a lump sum price for temporary chain-link fencing around the perimeter of the site. Temporary fencing shall be required for the life of the project until permanent fencing, and pedestrian hardscapes are installed.

4. Can the City provide any boring logs associated with the design development for the project?
    - Please refer to 'Exhibit B' within published Addenda No. 001 for all geotechnical explorations conducted within the project limits. The City does not intend to conduct additional explorations prior to the bid opening date.
5. Can the GDOT SPEC 1125 Headwalls be precast?
    - All headwalls are permitted to be precast structures.
6. Will a CAD file of the design be available to the Contractor?
    - The design CAD file will be available to the awarded contractor following signing of an Electronic File Release Agreement with the design firm.
7. The plans call for a top of retaining wall step of 12" in two places, and Redi-Rock modular retaining walls only come in 18" blocks at the smallest. Please clarify how to proceed given this information, and a sample cutsheet of the Redi-Rock retaining wall is incorporated in the plan set.
    - If the specified retaining wall block only comes is 18" vertical segments, top of wall elevations will need to be adjusted the 6" to accommodate the type of material being proposed. We foresee these top of wall adjustments as well as associated grading changes being addressed following the project award and upon receipt of stamped retaining design and retaining wall submittals.
8. Unsuitable soils allowance – What is to be considered as unsuitable soils and is rock excavation to be included?
    - Given that the site is mostly previously placed fill, this line item is incorporated for unexpected encounters of miscellaneous fill material such as large boulders, curb & gutter, woody type material that would potentially have to be hauled to a landfill to dispose of. Excessive topsoil, alluvial soils or non-structural soils will not fall within this pay item. These shall be included in excavation complete. This allowance does not cover rock removal, a rock removal situation is not anticipated given soil borings. In the event of a rock occurrence, a removal price will be negotiated at that time
9. What if unsuitable soils are encountered at retaining wall footing elevation?
    - If unsuitable soils are encountered in the retaining wall footing excavation, the remediation determination will be made at the time of encounter. The City's Geotechnical Engineer and retaining wall designer will collaborate to determine effective solution for subgrade remediation.
10. Will a lane drop be permitted for construction purposes?
    - A lane drop on Waugh St. and Thornton Ave will be permitted but provisions for rush hour traffic and school traffic will need to be made.
11. Will minutes from the Pre-Bid meeting be posted?
    - The city does not issue an official meeting minutes document from the pre-bid meeting, but Addenda No. 001 is intended to provide clarifications regarding the project that is not specifically stated otherwise in the original contract documents.

BY:

T. Jackson Sheppard, E.I.T.
Project Manager

EXHIBIT A
REVISED BID FORM

DALTON PROJECT NO. PW-2024-SP-171
PRATER ALLEY STORMWATER DETENTION PROJECT REVISED BID FORM

ITEM	DESCRIPTION	UNITS	QNTY	UNIT PRICE	TOTAL
GENERAL					
1	GENERAL CONDITIONS	LS	1		
2	PAYMENT & PERFORMANCE BONDING	LS	1		
3	CONSTRUCTION STAKING AND AS BUILT SURVEY	LS	1		
4	OWNER-DIRECTED WORK	LS	1	$ 20,000.00	$ 20,000.00
				SUBTOTAL	
DEMOLITION AND EARTHWORK					
5	CONCRETE SIDEWALK REMOVAL	SY	277		
6	CURB & GUTTER REMOVAL	LF	146		
7	ABANDON RCP IN PLACE	LF	35		
8	48" RCP REMOVAL	LF	143		
9	GRADING COMPLETE (INCL. DEWATERING OF EXCAVATION IF NEEDED)	LS	1		
				SUBTOTAL	
POND AND DRAINAGE SYSTEM					
10	RETAINING WALL (INCL. SELECT BACKFILL IF NEEDED)	LS	1		
11	HEADWALL (GDOT 1125)	EA	2		
12	24" CLASS III RCP	LF	19		
				SUBTOTAL	
SITE RESTORATION					
13	CONCRETE SIDEWALK	SY	145		
14	ORNAMENTAL FENCE	LF	900		
15	MAINTENANCE ACCESS GATE	EA	1		
				SUBTOTAL	
EROSION CONTROL					
16	RIPRAP (INSTALLED)	SY	250		
17	VEGETATED EMERGENCY SPILLWAY - TOPSOIL	SY	275		
18	VEGETATED EMERGENCY SPILLWAY - EROSION CONTROL BLANKET	SY	275		
19	TEMPORARY GRASS SEEDING	SY	4410		
20	TURF GRASS SEEDING	SY	2560		
21	BERMUDA SOD	SY	1,850		
22	SILT FENCE	LF	1,400		
23	FILTER RING	EA	2		
24	CONSTRUCTION EXIT	EA	1		
25	CURB INLET PROTECTION	EA	1		
26	TEMPORARY PUMP & FILTER BAG	EA	1		
27	TEMPORARY STORMWATER BYPASS SYSTEM	LS	1		
				SUBTOTAL	
MISCELLANEOUS					
28	TRAFFIC CONTROL	LS	1		
29	UTILITY CONFLICTS & TEMPORARY SERVICE	LS	1		
30	TEMPORARY FENCING	LS	1		
31	UNSUITABLE SOILS ALLOWANCE (REMOVAL)*	CY	1,000		
	*PROVIDE UNIT PRICE			SUBTOTAL	

Company Name:_____

Authorized Bid Rep. Signature:_____

Authorized Bid Rep. Title:_____

TOTAL

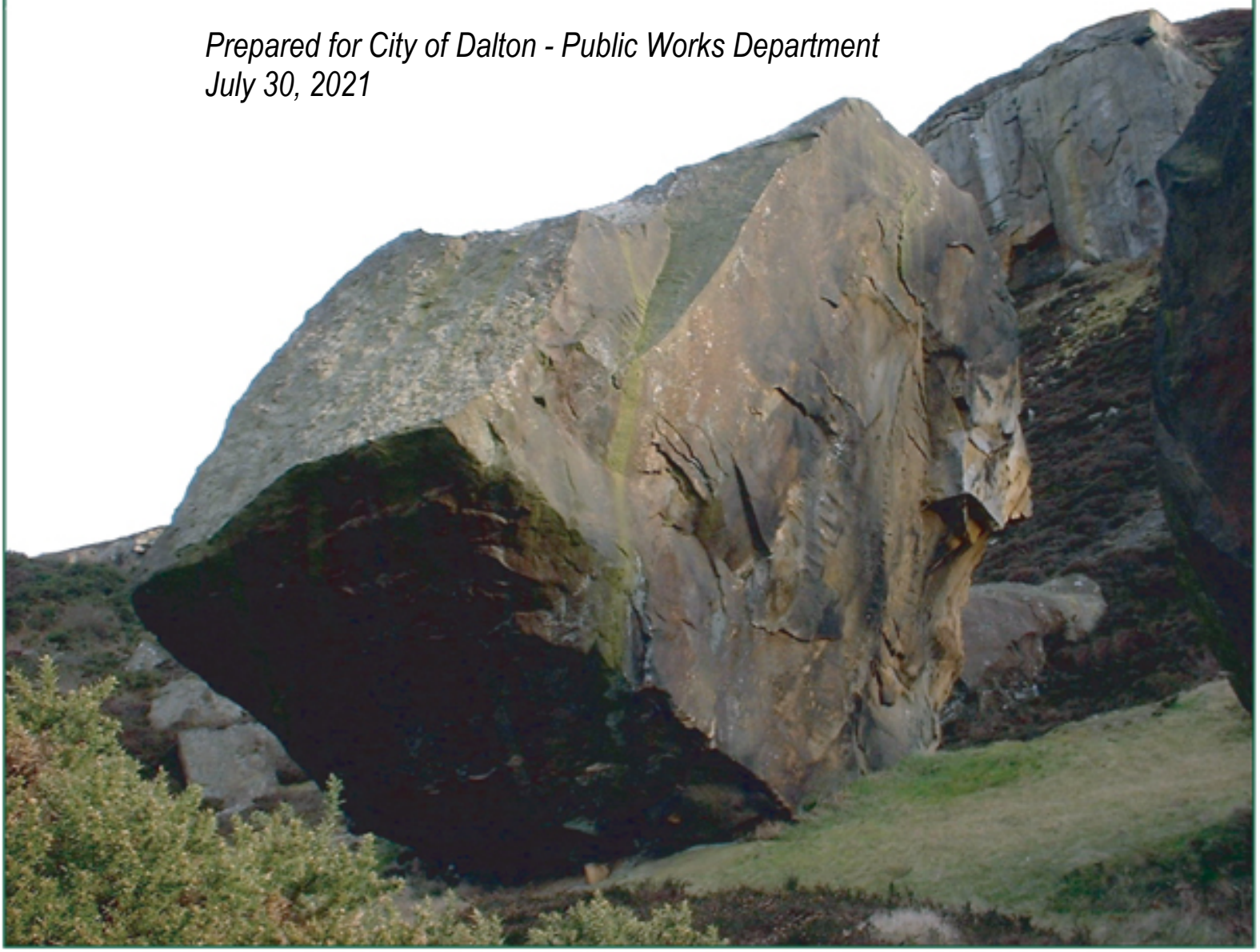
EXHIBIT B
SOIL BORING LOGS

GEO HYDRO ENGINEERS

Report of Subsurface Exploration and Infiltration Testing

Proposed Underground Stormwater Pond
Dalton City Hall
Dalton, Georgia
Project Number 210622.20

Prepared for City of Dalton - Public Works Department
July 30, 2021



Ms. Megan Elliott
Public Works Department
City of Dalton
535 Elm Street
Dalton, Georgia 30722

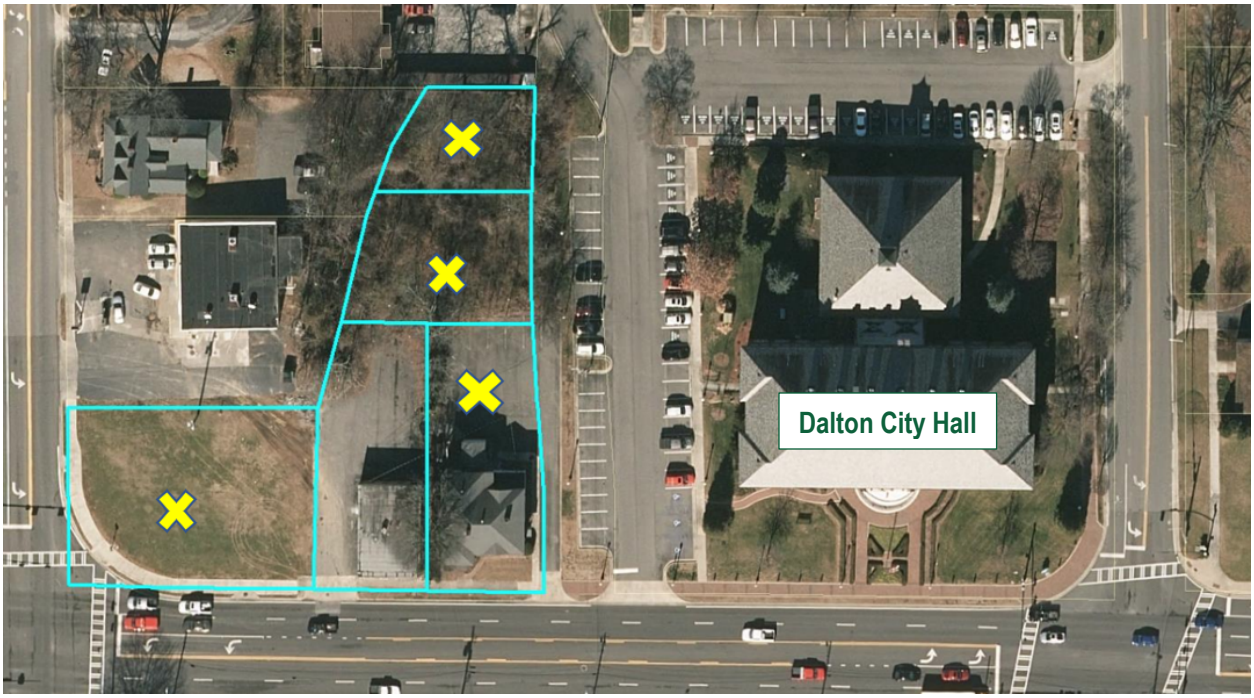
July 30, 2021

Report of Subsurface Exploration and Infiltration Testing
Proposed Underground Stormwater Pond
Dalton City Hall
Dalton, Georgia
Project Number 210622.20

Dear Ms. Elliott:

Geo-Hydro Engineers, Inc. has completed the authorized infiltration testing for the above referenced project. The scope of services was outlined in our proposal number 210622.P0 dated June 9, 2021.

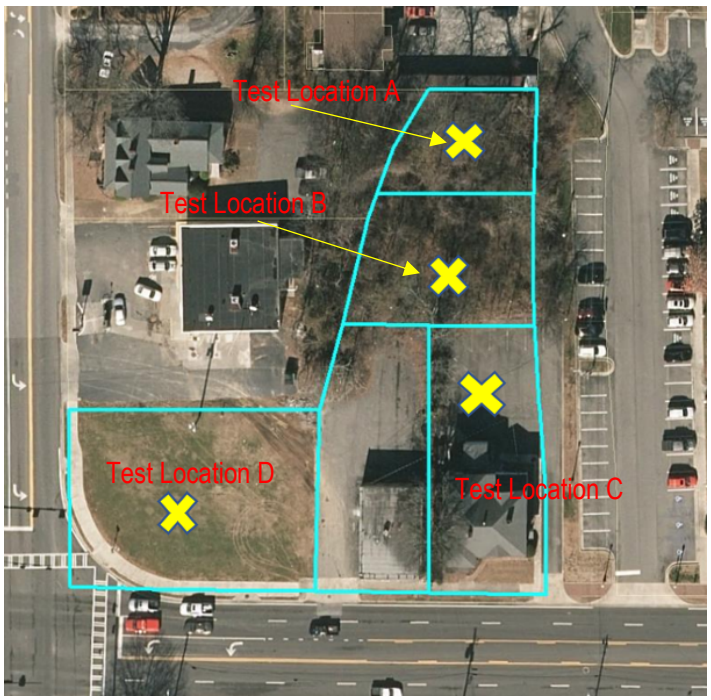
We understand that the City of Dalton is evaluating four lots west of City Hall for a possible underground stormwater detention pond. The lots include a surface parking lot and grassed and wooded areas. We understand that the pond will have an infiltration horizon approximately 5 feet below current grades, but the project is in a preliminary phase and a final footprint and elevations have not been determined at this time. The annotated aerial photograph below was provided to us and shows the total parcel property lines and requested test locations. At the time of our exploration the structure in the southeastern corner of the property had been demolished and the wooded lots to its north had been cleared.



1000 Cobb Place Boulevard, Suite 290 • Kennesaw, Georgia 30144
o: 770.426.7100 • f: 770.426.5209 • *www.geohydro.com*

GEO HYDRO ENGINEERS

Testing Procedures



We performed testing at four locations as shown on the annotated aerial photograph to the right. Figure 2 in the Appendix shows the individual test locations. At each test location, we performed one or two infiltration tests and attempted to advance an offset borehole to a depth of 10 feet to determine if groundwater was present within 10 feet of the ground surface.

The test holes were advanced to depths ranging from about 2 to 5 feet. Depending on access conditions, the boreholes were advanced using hand equipment producing a 4-inch diameter borehole or a drill rig producing an approximately 7½-inch diameter borehole. A layer of gravel was added to the bottom of the holes to protect the infiltration surface from scour during the tests. Water was then introduced to the hole and allowed to saturate the borehole for a minimum of four hours. Subsequently, the drop in water level was measured over time. Percolation testing was performed in accordance with the Modified Taft Engineering Center Method. The percolation rate was converted to a vertical infiltration rate by using the equation below.

Infiltration Rate = (Percolation Rate)/(Reduction Factor)

Where the Reduction Factor is given by:

$$Rf = (2d1 - \Delta d)/DIA + 1$$

With:

d1	=	initial water depth (in)
Δd	=	average/final water level drop (in)
DIA	=	diameter of the percolation test area hole (in)

GEO HYDRO ENGINEERS

INFILTRATION TEST RESULTS

Test Location A

Testing at Location A included two infiltration tests performed in boreholes advanced with hand equipment. The hand equipment encountered fill materials containing large rock fragments similar to #57 or #34 stone. The hand equipment encountered conditions causing auger refusal, which prevented them from reaching the requested test depth of 5 feet. The results of the infiltration testing are shown in the table below.

Infiltration Test	Borehole Diameter (inches)	Test Depth (feet)	Infiltration Rate (inches/hour)
I-1	4	2	0.18
I-2	4	4	0.04

Similarly, the offset boring to evaluate groundwater conditions in the area was unable to penetrate beyond a depth of 4 feet due to large rock fragments within the existing fill.

Test Location B

Testing at Location B included two test holes advanced to a depth of 5 feet with hand equipment. An offset borehole was advanced with hand equipment to evaluate groundwater conditions. All three test holes advanced at Test Location B encountered groundwater at a depth of 1 foot or less. Due to the shallow groundwater conditions encountered at Test Location, infiltration testing could not be performed.

Infiltration Test	Borehole Diameter (inches)	Test Depth (feet)	Infiltration Rate (inches/hour)
I-6	4	5	N/A (Groundwater @ 0.5')
I-7	4	2	N/A (Groundwater @ 0.5')
I-8	4	5	N/A (Groundwater @ 1')

Test Location C

Testing at Location C included two test holes advanced to a depth of 5 feet by drilling equipment. The test holes advanced at Test Location C encountered groundwater at the depths shown in the table below. Due to the shallow groundwater conditions encountered at Test Location, infiltration testing could not be performed in tests I-3 and I-4.

Infiltration Test	Borehole Diameter (inches)	Test Depth (feet)	Infiltration Rate (inches/hour)
I-3	7½	5	N/A (Groundwater @ 4.5')
I-4	7½	5	N/A (Groundwater @ 4.5')

At Test Location C, an offset boring (B-2) was advanced to a depth of 10 feet using a drill rig. At the completion of our field work, groundwater was encountered in boring B-2 at a depth of 9 feet. It is important to note that groundwater measured at the time of drilling is typically lower than stabilized groundwater levels. Based on our experience, the groundwater level at Test Location C is likely above the levels measured in infiltration test holes I-3 and I-4.

Test Location D

Testing at Location D included two test holes advanced to a depth of 5 feet by drilling equipment. Both test holes advanced to a depth of 5 feet at Test Location C encountered groundwater at the depths shown in the table below. Due to the shallow groundwater conditions encountered at Test Location, infiltration testing could not be performed in tests I-9 and I-10.

An additional test hole was advanced to a depth of 2 feet using hand equipment at test location I-5. The table below shows the results of test I-5.

Infiltration Test	Borehole Diameter (inches)	Test Depth (feet)	Infiltration Rate (inches/hour)
I-5	4	2	0.25
I-9	7½	5	N/A (Groundwater @ 4.5')
I-10	7½	5	N/A (Groundwater @ 4.5')

At Test Location D, an offset boring (B-1) was advanced to a depth of 10 feet using a drill rig. At the completion of our field work, groundwater was encountered in boring B-2 at a depth of 5 feet. It is important to note that groundwater measured at the time of drilling is typically lower than stabilized groundwater levels.

Additional Geotechnical Considerations

The following evaluations and recommendations are based on the information available on the proposed construction, the data obtained from the test borings, infiltration tests, and our experience with soils and subsurface conditions similar to those encountered at this site. Because the test holes represent a very small statistical sampling of subsurface conditions, it is possible that conditions may be encountered during supplemental explorations or construction that are substantially different from those indicated by the test borings. In these instances, adjustments to the design and construction may be necessary.

Fill materials were encountered in all of the borings across the site. Boring B-1 was terminated at a depth of 10 feet without penetrating the fill materials. Existing fill materials may be encountered in areas of the site not directly explored by the test borings. Any fill materials encountered during construction should be thoroughly evaluated, and any loose or unstable fill materials should be excavated and replaced with structural fill if they cannot be adequately densified in place.

Additionally, the infiltration properties of fill materials are typically lower than residual soils and can approach zero if fill materials are placed wet of the optimum moisture content as determined by a standard Proctor test (ASTM D698).

At the time of our exploration, groundwater was encountered 9 of the 12 test holes advanced for the project at depths ranging from about 6 inches to 9 feet. The depth to groundwater varies drastically across the site. A piped creek was observed running north off the properties. We expect the groundwater level to be encountered around the same elevation as the creek on site.

Based on the results of our exploration and testing, we do not expect that the subsurface soils within the horizon of the upper 5 feet will provide appreciable infiltration.

* * * * *

We appreciate the opportunity to serve as your geotechnical consultant for this project and are prepared to provide any additional services you may require. If you have any questions concerning this report or any of our services, please call us.

Sincerely,

GEO-HYDRO ENGINEERS, INC.

John T. Redding, E.I.T.
Staff Engineer
jredding@geohydro.com

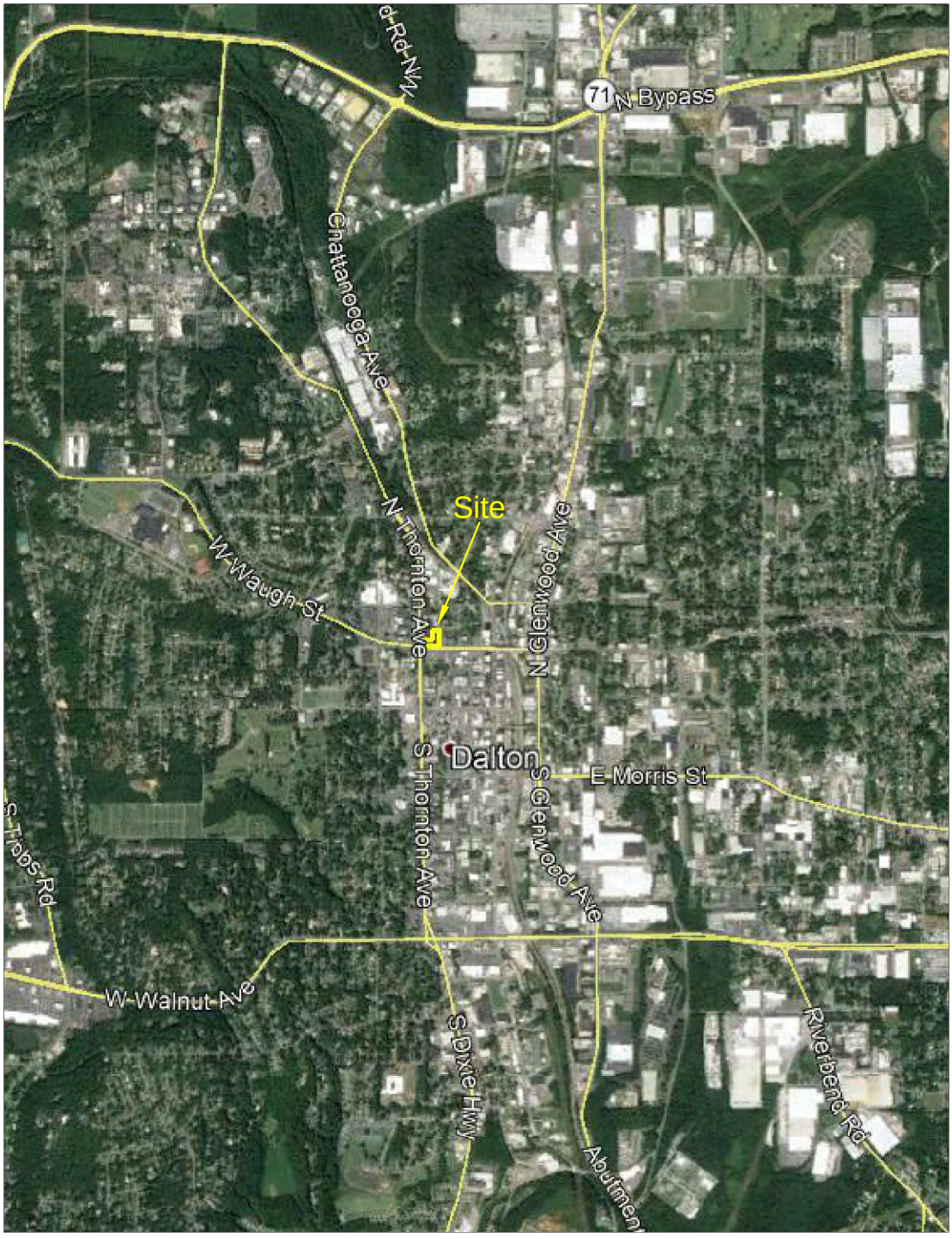
A. Marty Peninger, P.E.
Senior Geotechnical Engineer
mpeninger@geohydro.com

GEORGIA REGISTERED No. 35685 PROFESSIONAL ENGINEER A. MARTY PENINGER

JTR/AMP/210622.20 - Underground Stormwater Pond - Dalton City Hall

GEO HYDRO ENGINEERS

APPENDIX

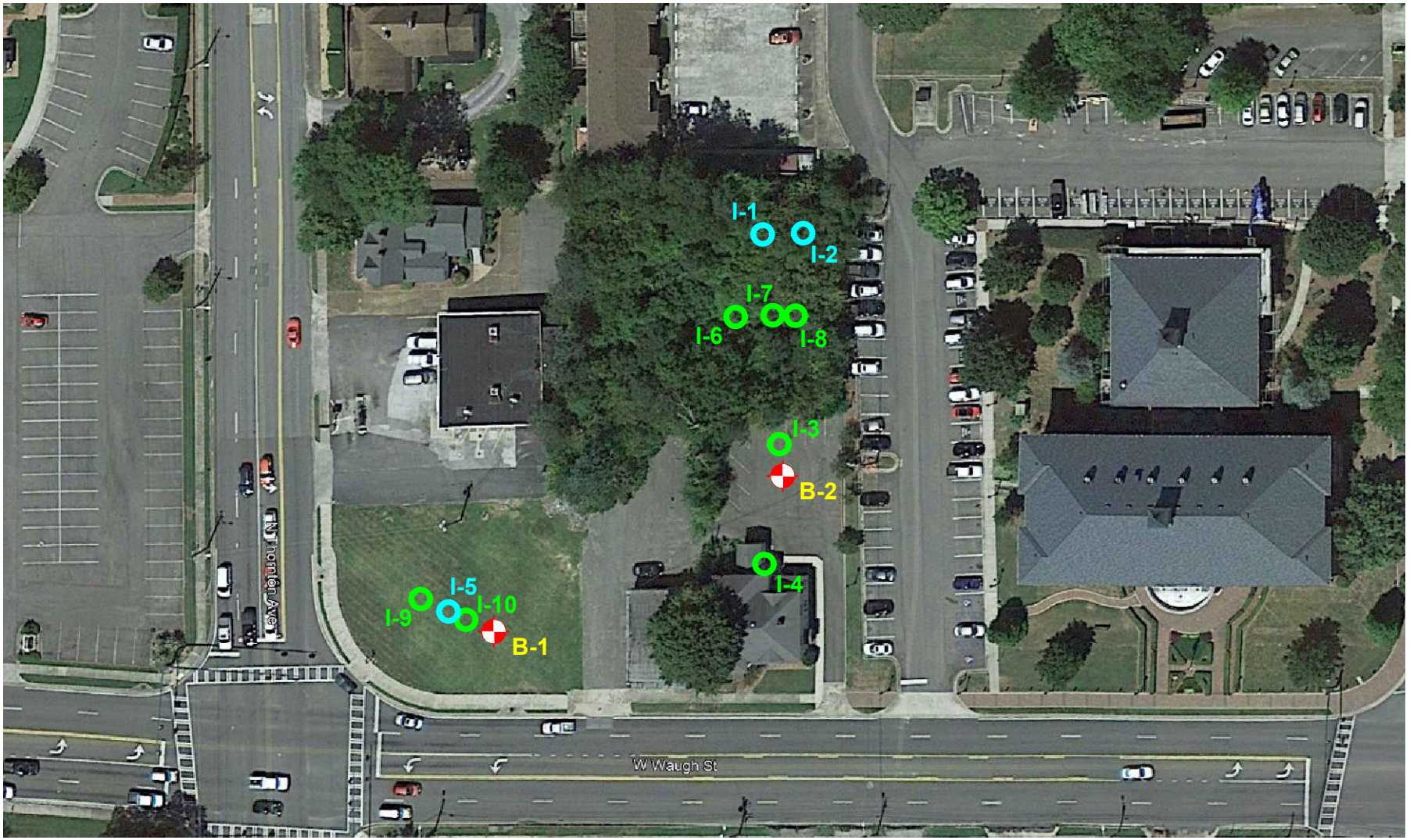


0 0.25 0.5 1 1.5

Approximate Scale, Miles

Figure 1: Site Location Plan	Proposed Underground Stormwater Pond Dalton City Hall Dalton, Georgia Project Number 210622.20

GEO HYDRO ENGINEERS



0 40 80 160 240

Approximate Scale: 1"=80'

LEGEND:
- Soil Test Boring
- Infiltration Test Location
- Test Hole - Groundwater Encountered

Figure 2: Boring Location Plan

Proposed Underground Stormwater Pond
Dalton City Hall
Dalton, Georgia
Project Number 210622.20

Symbols and Nomenclature

Symbols

▌	Thin-walled tube (TWT) sample recovered
▯	Thin-walled tube (TWT) sample not recovered
●	Standard penetration resistance (ASTM D1586)
50/2”	Number of blows (50) to drive the split-spoon a number of inches (2)
65%	Percentage of rock core recovered
RQD	Rock quality designation - % of recovered core sample which is 4 or more inches long
GW	Groundwater
	Water level at least 24 hours after drilling
	Water level one hour or less after drilling
ALLUV	Alluvium
TOP	Topsoil
PM	Pavement Materials
CONC	Concrete
FILL	Fill Material
RES	Residual Soil
PWR	Partially Weathered Rock
SPT	Standard Penetration Testing

Penetration Resistance Results	Number of Blows, N	Approximate Relative Density
Sands	0-4	very loose
	5-10	loose
	11-20	firm
	21-30	very firm
	31-50	dense
	Over 50	very dense

	Number of Blows, N	Approximate Consistency
Silts and	0-1	very soft
Clays	2-4	soft
	5-8	firm
	9-15	stiff
	16-30	very stiff
	31-50	hard
	Over 50	very hard

Drilling Procedures

Soil sampling and standard penetration testing performed in accordance with ASTM D 1586. The standard penetration resistance is the number of blows of a 140-pound hammer falling 30 inches to drive a 2-inch O.D., 1.4-inch I.D. split-spoon sampler one foot. Rock coring is performed in accordance with ASTM D 2113. Thin-walled tube sampling is performed in accordance with ASTM D 1587.

B-1

Test Boring Record

GEO HYDRO ENGINEERS

Project: **Proposed Underground Stormwater Pond**		Project No: **210622.20**
Location: **Dalton, Georgia**		Date: **7/21/21**
Method: **HSA- ASTM D1586**	GWT at Drilling: **7 feet**	G.S. Elev:
Driller: **GCD (Rope and Cathead)**	GWT at 6 hrs: **5 feet**	Logged By: **JTR**

Elev. (Ft)	Depth (Ft)	GWT	Symbol	Description	N	Standard Penetration Test (Blows/Foot) 0 10 20 30 40 50 60 70 80 90 100
				Stiff to very stiff red and brown fine sandy silt (ML) (FILL)		
					10	
	5	▼			11	
		▽			13	
	10			Boring Terminated at 10 feet	17	
	15					

Remarks: Boring backfilled after final groundwater check

TEST BORING RECORD BORING LOGS.GPJ GEO HYDRO.GDT 7/27/21

B-2

Test Boring Record

GEO HYDRO ENGINEERS

Project: **Proposed Underground Stormwater Pond**		Project No: **210622.20**
Location: **Dalton, Georgia**		Date: **7/21/21**
Method: **HSA- ASTM D1586**	GWT at Drilling: **Not Encountetered**	G.S. Elev:
Driller: **GCD (Rope and Cathead)**	GWT at 6 hrs: **9 feet**	Logged By: **JTR**

Elev. (Ft)	Depth (Ft)	GWT	Symbol	Description	N	Standard Penetration Test (Blows/Foot) 0 10 20 30 40 50 60 70 80 90 100
				Asphalt (Approximately 3 inches)		
				Crushed Stone Base (Approximately 12 inches)		
				Large stone fill (Approximately 2 feet)		
				Stiff to very stiff tan to brown silty clay (CL) (RESIDUUM)		
	5				15	
					22	
		▼		Very form brown silty fine sand (SM)		
	10			Boring Terminated at 10 feet	23	
	15					

Remarks: Boring backfilled after final groundwater check

TEST BORING RECORD BORING LOGS.GPJ GEO HYDRO.GDT 7/27/21

EXHIBIT C
Official Plan Holder's List

City of Dalton Public Works Department
Mandatory Pre-Bid Meeting - PRATER ALLEY STORMWATER DETENTION PROJECT
Monday, March 18, 2024 - 1:00 PM
Sign-In Sheet

Name	Company	Phone	Email (Project Addenda will be sent to this address)
T. Jackson Sheppard	City of Dalton - PW	706-278-7077	jsheppard@daltonga.gov
Chad Townsend	City of Dalton	706-278-7077	ctownsend@daltonga.gov
Robbie Greene	Summers - Taylor	423-618-1584	robbieg@Summerstaylor.Com
Lee Hamrick	Simpson Construction A Division of Summers-Taylor	(423)716-5693	leeh@summerstaylor.com
Rodney Lock	Lock's Dozing, Inc.	706-463-1272	Rlock@locksdozing.us
Kevin Epps	International waste Service	404-965-6389	
Lou Bennett	Integrated Site work	770 881 6138	lou@integratedsitework.com
Jeremy Reed	B+J Reed	706-463-2009	
AJ Williams	International Waste Service	678-519-8000	alvin.intlwaste@gmail.com